# Combating Accidental Microbial Episodes by Back-Ground Chlorine Residuals in a Scaled-up Distribution Network (Rig) Using a Central Composite Design (CCD)

S. Rasheed, I. Hashmi, Q. Zhou, J. K. Kim, and L. C. Campos

Abstract—A quadratic model (p < 0.0001) was developed by using a central composite design of 50 experimental runs (42 non-center + 8 center points) to assess the efficiency of background chlorine residuals in combating accidental microbial episodes in a scaled-up distribution network (DN) (rig). A known amount of background chlorine residuals were maintained in the DN and a required number of bacteria, Escherichia coli K-12 strain, was introduced by an injection port in the pipe-loop system. Samples were taken at various time intervals at different pipe lengths. A spread-plate count was performed to count the bacterial number. Microbial concentration and time (p < 0.0001), pipe length (p < 0.022), background chlorine residuals (p < 0.07) and time<sup>2</sup> (p < 0.09) were observed as significant factors. The model that was developed was significant. The ramp function of variables shows that, at the microbial count of 10<sup>6</sup>, at 0.76 L/min, with a pipe length of 133 meters, a back ground residual chlorine of 0.16mg/L was enough for the complete inactivation of a microbial episode in approximately 18 minutes.

*Index Terms*—Central composite design (CCD), distribution network, Escherichia coli, residual chlorine.

### I. INTRODUCTION

The purpose of a water supply-distribution system is to deliver safe, potable water which is adequate in quantity and acceptable in terms of taste, odor and appearance [1]. Surface run-off, cross-connection and the leakage of sewage disposal systems as well as septic tanks' wrecked or leaking pipes, back siphonage from a plumbing fixture or cross-connection into a water supply line and intermittent water supply are some other reasons behind the bacterial nemesis of the drinking-water industry, especially in developing countries [2]. These external contamination events can act as a source of inoculum, introducing nutrients and resulting in the decrease of residual disinfectant concentrations within the distribution system, causing degradation of water quality. A poorly maintained distribution system can act as a vehicle for

[3]. Nowadays, preserving the water quality throughout the water distribution system is therefore the most challenging technological issue. An important mitigation measure that can be employed to protect against intentional or accidental microbial intrusion/contamination is the maintenance of suitable chlorine residuals throughout the distribution network, often regarded as "residual maintenance strategy" [4]. Chlorine provides the residual barrier the distribution system worldwide. The recommended chlorine residual for water, that is centrally treated, at the point of delivery should fall within 0.2-0.5 mg/l [5]. This residual chlorine concentration reduces the risk of general contamination by the accidental entry of microorganisms due to either back siphonage or to a re-growth of the microorganism within the distribution network as breakage from the biofilm [6]. Thus, controlling this concentration in drinking water is a very important aspect, since the decrease in its level below that recommended may cause a secondary development of microorganisms [7]. Changes in chlorine residuals can be used as indicators of microbial contamination [8]. Experience has shown that the maintenance of the chlorine residual cannot be relied upon to totally prevent the occurrence of bacteria. As water flows from the treatment plant to the consumer's tap, the water quality deteriorates because of a decreasing residual chlorine concentration, especially for 'long residence' times. It was observed that, as the chlorine residual decreased from 4.6 ppm at the plant to 0.2 ppm at the household, there was a statistically significant increase in total and thermo tolerant coliforms [4]. Although chlorine residuals greatly contribute to the inactivation and re-growth of indicator bacteria, i.e. faecal coliforms in the pipeline, the question awaiting an answer, is the level of inactivation at the recommended levels of chlorine residual by the World Health Organization (W.H.O) and the effect of environmental factors upon the efficiency of background chlorination and the required time to combat the microbial attack [9]. Furthermore, the ability of disinfectant residuals to inactivate microorganisms between the time that they enter the distribution system and the time they reach the consumer is still to be analyzed. So the present study was designed to quantitatively assess a distribution system's vulnerability against microbial intrusion and to evaluate the efficiency of background residual chlorine in combating any accidental microbial event from occurring in the system and the factors that contribute towards its failure in connection with the

pathogens transmission and may even contribute

significantly to gastrointestinal diseases in the community

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microbial episode.

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#### II. MATERIALS AND METHOD

# A. Design of Experiment (DoE) by the Central Composite Design (CCD)

With this technique, the main objective is to optimize the response surface that is influenced by various process parameters. It also quantifies the relationship between the controllable input parameters and the obtained response surfaces [10]. Design Expert (Trial Version 9, MN, USA) was used as the tool for the Design of the Experiment (DoE). The objective of DoE is the selection of the points where the response should be evaluated. A CCD consisting of a full factorial design with 30 experiments was selected to simultaneously optimize the levels of these variables in attaining the best system performance. The independent variables were: pipe length, microbial concentration, flow rate, background chlorine residuals and time, as depicted in Table I. The lower values are coded as -1 and the higher values are coded as +1.

TABLE I: INDEPENDENT VARIABLES AND THEIR LOW/ HIGH-LEVEL VALUES BY THE CCD IN THE DESIGN EXPERT

Coded Values		-α	-1	0	+1	+α
<u>Variables</u>		Lowest	Low	Centre	High	Highest
Pipe Length	A	22.5	67.5	112.5	157.5	202.5
Microbial Conc.	В	10^5	10^6	10^7	10^8	10^9
Flow Rate	C	0	0.5	1	1.5	2
Backgrnd. Chlorine	D	0.05	0.15	0.25	0.35	0.45
Time	Е	2.5	15	27.5	40	52.5

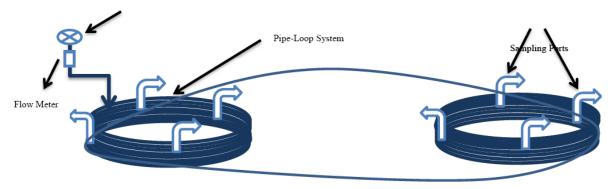
### B. The Distribution Rig

A laboratory-scale distribution network (proto-type rig) was established consisting of HDPE pipe of 220 meters in two concentric loops and a main water reservoir with a flow meter to regulate the flow within the network. Nine sampling ports were provided (22.5 meters apart) in order to collect the samples at various time intervals (Fig. 1). The loop network is different from a single pipe and a parallel pipe. The number of paths, the water flow from water sources to the observatory point, depends on the amount of looping. De-chlorinated tap water was used (by employing an injection syringe) as the medium to introduce a bacterial event.

Commercial sodium hypochlorite(10.5%) was freshly prepared and different dilutions were applied as per the experimental requirements. For the disinfection of the suspended bacteria, E. coli, K-12 strains were grown on agar spread plates at 37  $^{\circ}$ C for 24 hours. The bacterial culture was washed twice with phosphate buffer and centrifuged at 4,500g to get rid of any agar remnants. A measured quantity of 10<sup>6</sup>, 10<sup>7</sup>, 10<sup>8</sup> and 10<sup>9</sup> was introduced through an injection port when a required background chlorine level was maintained. The flow rate was uniformly kept by using a flow meter and samples were collected at various pipe lengths. The residual chlorine was quenched by adding 0.01 N sodium thiosulphate (Na<sub>2</sub>S<sub>2</sub>O<sub>3</sub>) to cease further disinfection reaction. Serially-diluted sub-samples of each suspension were plated as spread-plate counts and incubated for 24 hours at 37°C. The 'free' chlorine was measured with a Spectroquant Picco colorimeter according to standard methods [11].

### III. RESULTS AND DISCUSSION

The main objective was to evaluate the efficiency of background chlorine in combating an accidental or a deliberatelyinduced microbial event in a scaled-up distribution network. ANOVA analysis was performed and a quadratic model was developed with a significant p value < 0.0001. Values of 'Prob> F' less than 0.05 indicate model terms are significant and values greater than 0.1indicatethe model terms are not significant [12]. The terms A, B, D and E are significant with p < 0.022, 0.0001, 0.07 and 0.0001 respectively. The coefficient of determination(R2) is 81%, ensuring a satisfactory agreement of the quadratic models to the experimental data. Predicted R-square of 0.5015 is in reasonable agreement with Adj. R- square of 0.6948 i-e., the difference is less than 0.2. Adeq. Precision measures signal to noise ratio. A ratio greater than 4 is required which is 10.178 in this case i.e., the model may be used to navigate the signal. PRESS is a measure of how well the model predicts the responses in new experiments. Small PRESS is recommended [13]. The lack of fit was observed insignificant (p=0.842) which describes the variation in the data around the fitted model. If the model does not fit the data well, the lack of fit will be significant, i-e., p < 0.05. (Table II).



 $Fig. 1. \ Layout \ of \ the \ Prototype \ distribution \ network \ (rig).$ 

# A. Effect of Various Back Ground Chlorine Concentration (BGCC)

Various background chlorine levels were analyzed, as given in Table I. Observing Fig. 2, it is evident that, with

higher levels of background chlorine (BGC), more disinfection is achieved at a given time and distance in the loop system. The BGCL of 0.05 achieved the minimum disinfectionwhile 0.45 mg/L achieved the maximum

efficiency of inactivation, approximately 28 minutes after the intrusion episode. It is clear that, for efficient disinfection, a large amount of BGCL is required and still takes almost one-half hour of reaction time. The 3-D surface plots and contour graphs provide a method to visualize the relationship between responses and experimental levels of each variable and the type of interactions between two test variables (Fig. 3 and Fig. 4). The results are in accordance with Gagnon *et al.*,

[14] who recommended a free chlorine residual of 0.6 mg/L to control bacterial growth in distribution systems. So it may inferred that a disinfectant residual can guard against only a small amount of reintroduced pathogens, and may easily be overcome by high concentrations of be inferred that a disinfectant residual can only guard against a small amount of re-introduced pathogens and may be easily overcome by high concentrations of contaminants.

	TABLE II: ANALYSIS OF VARIANCE	(ANOVA) FOR MICROBIAL DISINFECTION BY	BACKGROUND CHLORINATION
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Source	Sum of Squares	Df	Mean Square	F Value	P Value Prob. >F
Model	420.25	20	21.01	8.53	< 0.0001
A-Pipe Length	10.71	1	10.71	5.06	0.022
B-Microbes	232.15	1	232.15	72.67	< 0.0001
C-Flow Rate	0.52	1	0.52	0.16	0.6891
D-Background Cl	10.90	1	10.90	3.41	0.0749
E-Time	114.36	1	114.36	35.8	< 0.001
AB	8.08	1	8.08	2.53	0.1225
AC	0.66	1	0.66	0.21	0.6535
AD	5.29	1	5.29	1.66	0.2084
AE	1.29	1	1.29	0.40	0.5297
BC	0.090	1	0.090	0.028	0.8680
BD	0.028	1	0.028	8.195E-003	0.9254
BE	0.32	1	0.32	0.10	0.7524
CD	6.73	1	6.73	2.11	0.1575
CE	0.74	1	0.74	0.23	0.6340
DE	2.16	1	2.16	0.68	0.4179
A2	0.65	1	0.65	0.20	0.6541
B2	0.014	1	0.014	4.241E-003	0.9485
C2	1.07	1	1.07	0.33	0.5675
D2	4.68	1	4.68	1.47	0.2358
E2	9.43	1	9.43	2.95	0.0964
Residual	92.64	29	3.19		
Lack of Fit	59.97	22	2.73	0.58	0.8421
Pure Error	32.67	7	4.67		
Corr. Total	512.90	49			
Std Dev.	1.79	Mean	11.83	C. V %	15.11
R-Square	0.8194	Adj. R square	0.6948	Pred. R Square	0.5015
PRESS	255.67			Adeq. Precision	10.178

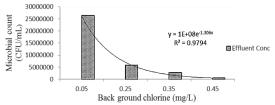


Fig. 2. Effect of various background chlorine residuals on accidental microbial intrusion.

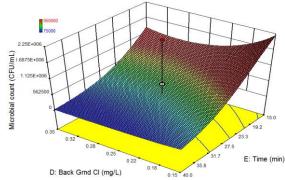


Fig. 3. 3-D response surface graph showing the effect of back ground chlorination with time.

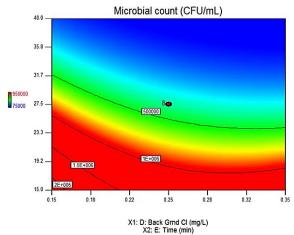


Fig. 4. Contour graph of back ground chlorine with time.

### B. The Effect of Pipe Length

The distance travelled by the microbial plume was observed to see the effect of WHO limit of 0.25mg/L BGCL in a laminar flow. The graph shows that more the distance covered by the plume, more the disinfection achieve

dirrespective of no mixing of water in the pipe (See Fig. 5).

The long distance is indirectly providing the contact time between the intruder microbes and the back ground chlorine resulting in efficient disinfection. Similar results are also mentioned by other researchers [15].

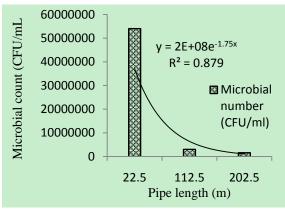


Fig. 5. Decrease of microbial count with increasing pipe.

### C. The Effect of Flow Rate

Various flow rates were analyzed to observe their effect on microbial inactivation in a distribution network. The flow rate indirectly related to the contact time between microbes and background chlorine.

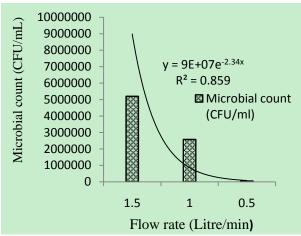
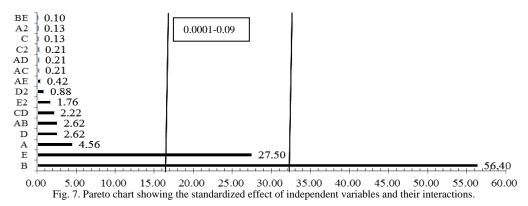


Fig. 6. Effect of flow rate on microbial inactivation.

The more time that it takes water to reach the sampling point, more is the contact time available for background chlorine to complete the disinfection reaction (Fig. 6).

Ln (CFU/mL) =11.90 - 0.66 A +2.32B - 0.11C-0.50 D +1.62E+0.50AB+0.14AC+0.20AE+0.053BC-0.0-0.10BE+0.46CD-0.15CE-0.26DE-0.11A<sup>2</sup>-0.01630B<sup>2</sup> +0.14C<sup>2</sup>+0.29D<sup>2</sup>-0.41E<sup>2</sup> (1)



The most important factors which contribute to the disinfection of microbial episodes in the distribution network are presented in the Pareto chart shown in Fig. 7. Among all factors, the "number of microbes" (B) with 56.40% of the total factors' contribution, contributes the most towards the back ground chlorine's failure to combat the accidental event.

Then "time" (E), "pipe length" (A) and "background chlorine" (D) are influential in their efficiency of a distribution network's disinfection besides AB, CD, E<sup>2</sup> and D<sup>2</sup>respectively.

The model's terms given in Table III are summarized in the form of Equation 1 as follows:

The Eq. 1 reduces to Eq. 2 based on p values depicted in Response Table III, as given under:

Ln (CFU/mL) = 
$$11.90 - 0.66 \text{ A} + 2.32\text{B}$$
  
-  $0.11\text{C} - 0.50 \text{ D} + 1.62\text{E} - 0.41\text{E}^2$  (2)

The equation describes the effect of most influential factors very well, depicting the microbial episode as the main factor in the failure of distribution system and a main causative agent in the spread of water borne diseases. The other factors are pipe length, flow rate and background chlorine residuals and time.

TABLE III: RESPONSE/INTERACTION OF VARIOUS VARIABLES WITH P VALUES Response Intercept E^2 Ln(Micrba on). 11 8981 -0.657201 2.31509 -0.109756 -0.501753 -1.62492 -0.411965 0.0220 < 0.0001 0.6891 0.0749 < 0.0001 0.0964 Residual hoie.. 0.1462 -0.0113191 0.00262067 0.00401853 0.0522525 0.0191442 p= 0.3406 0.8245 0.7340 < 0.0001 0.1104 Legend .01<= p <.05 .05<= p <.10 p >=.10 p < .01

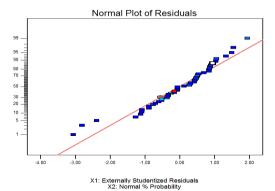


Fig. 8. Normal probability plot of residuals vs actual.

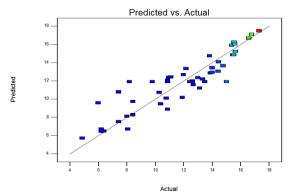


Fig. 9. Plot of predicted vs actual values by applied model.

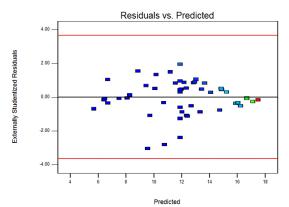


Fig. 10. Plot of predicted vs actual values by applied model.

### IV. VALIDATION OF THE MODEL

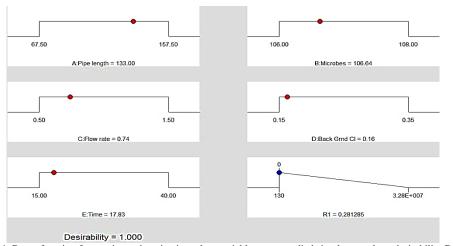
Graphical and numerical methods, as a primary tool and confirmation for graphical techniques were used to validate the model described by [16]. The normal probability plots of the residuals and the plots of the actual versus the predicted response in Fig.8and Fig. 9 respectively, revealing that the residuals generally fall on a straight line showing that experimental data is in full agreement with the predicted values. The predicted values were found to be statistically similar to the actual measured values, based on the plotted probability plot. It also implies that the errors are distributed normally. So the developed model was considered to be accurate and reliable for predicting the inactivation efficiency of background chlorine residuals against accidental microbial episode. In Fig. 9, all points are scattered around the straight line showing a pattern and no unusual shape. This implies that the model proposed is adequate to predict the TTHMs formation with various precursors discussed. The plot of residuals vs predicted plot that the experimental values and model predicted values fits very well (Fig. 10).

## V. NUMERICAL OPTIMIZATION

To optimize the level of each factor for maximum response "numerical optimization" process was employed. It is the combination of different optimized parameters, which gave maximum response it finds one or more points in the factor's domain that would maximize this objective function. The numerical optimization process involves combining the goals into an overall desirability function(D), an objective function that ranges from zero outside of the limits to one at the goal [12].Here the main objective was to evaluate the efficiency of background chlorination to inactivate microbial intrusion in a scaled-up distribution rig (network).

The ramp of variables shows that, at the microbial count of  $10^6$ , at 0.76 L/min and background residual chlorine of 0.16mg/L and a pipe length of 133 meters, the complete inactivation can be achieved in 18 minutes. The D =1 gives the maximum inactivation at the given conditions depicted in the ramp function in Fig. 11.

The present study and mathematical model gave a good measure of the effect of non-point source contamination on water quality and efficiency of back ground chlorine residuals to combat the accidental intrusion of microbes. More studies are required in this regard to monitor and manage the accidental contamination and combating strategies to provide safe drinking water to the community.



 $Fig. \ 11. \ Ramp \ function \ for \ maximum \ inactivation \ when \ variables \ were \ studied \ simultaneously \ at \ desirability \ D=1.$ 

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Ms. Rasheed won 5000 indigenous scholarship under higher education commission-Pakistan (HEC-Pak) and one year foreign scholarship under international research support initiative Programe under HEC-Pakistan (IRSIP-HEC-Pak) for university college London (UCL), UK in the field of drinking water chlorination, chlorine decay and subsequent trihalomethane formation and their removal through granulated activated carbon (GAC) and sand dual media. The further research is in progress.